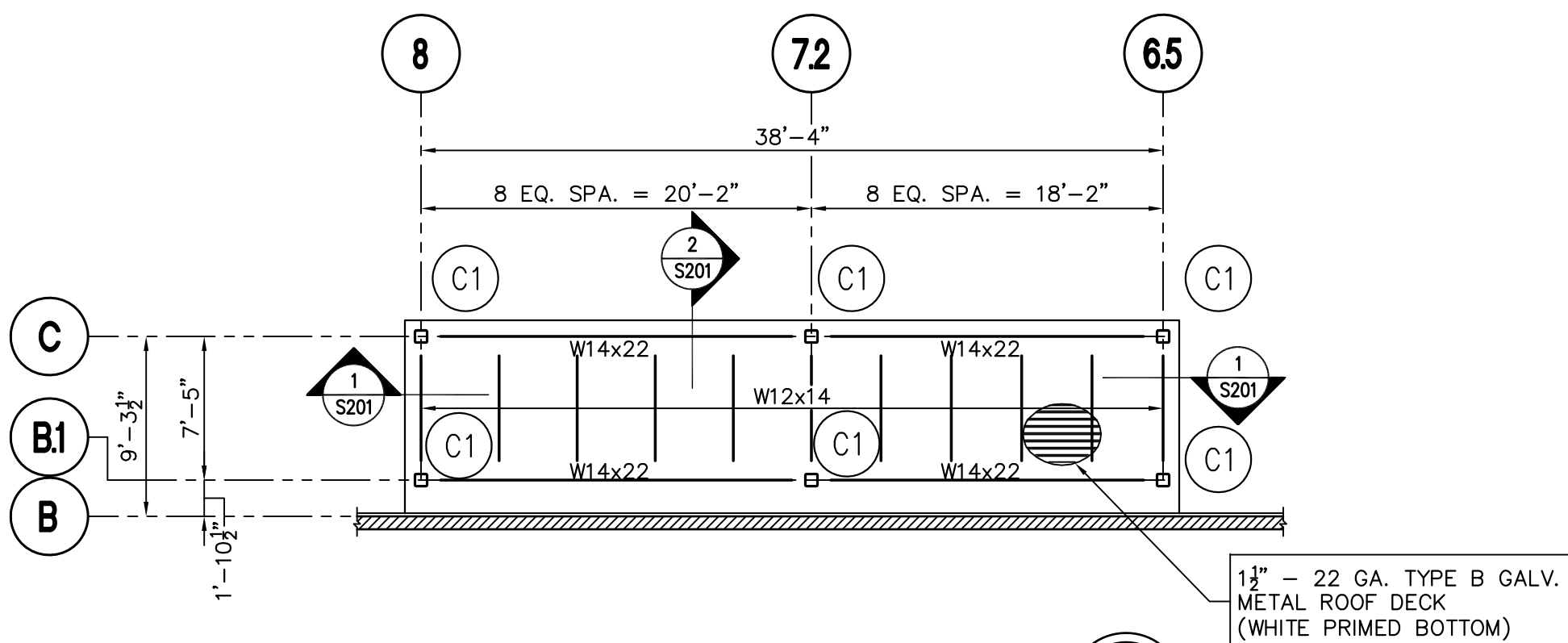
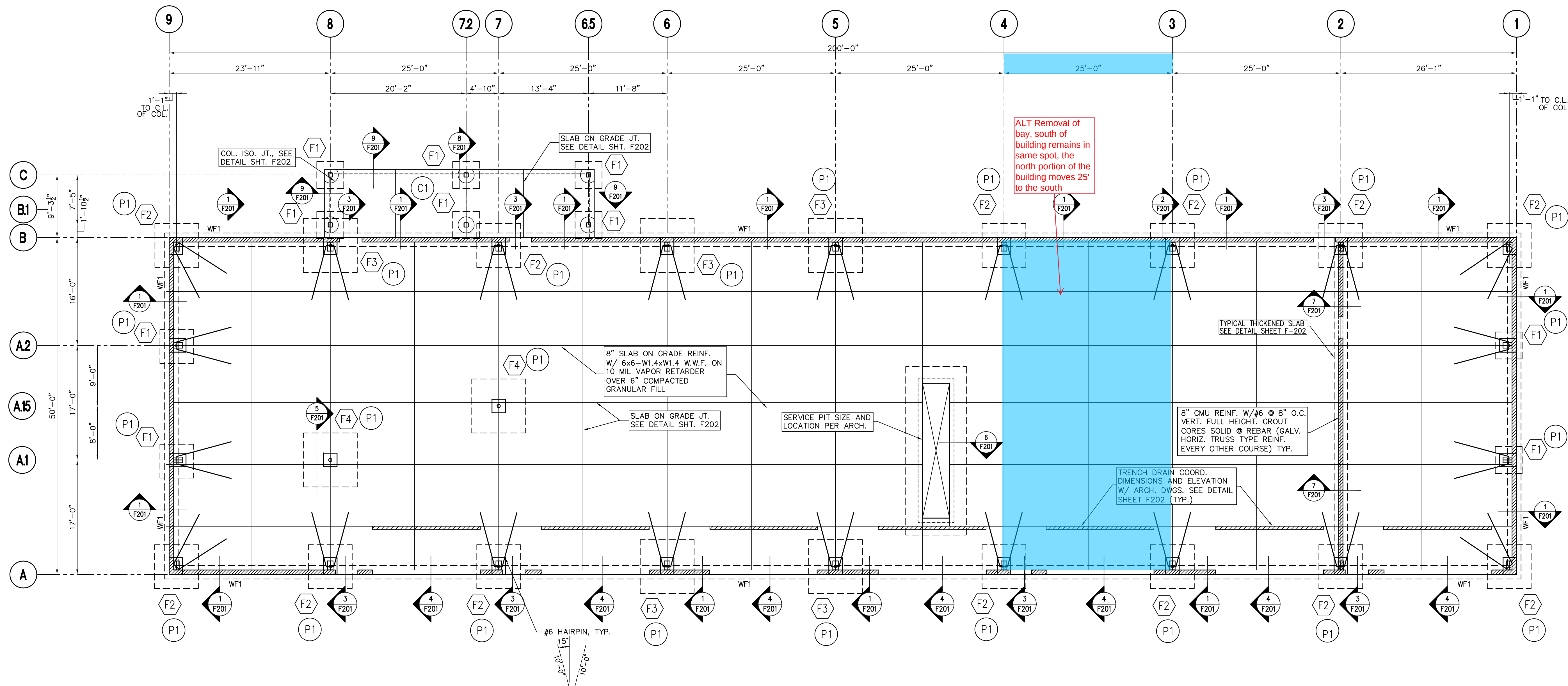




FOUNDATION PLAN
SCALE: 1/8"=1'-0"
NOTES:
1). TOP OF FINISH FLOORING ELEVATION = 100'-0" U.N.O.
2). TOP OF EXTERIOR FOOTING ELEVATION = 98'-8" U.N.O.
3). SEE ARCH. DWGS. FOR LOCATION OF DOOR OPENINGS AND SIZES.
4). TOP OF PEDESTAL ELEVATION 100'-0"
5). CENTER FTG. UNDER PEDESTAL U.N.O.

COLUMN FOOTING SCHEDULE		
MARK	SIZE (WxD)	REINFORCEMENT
F1	4'-0" x 4'-0" x 2'-0"	(4) - #5 E.W. BOT.
F2	6'-6" x 6'-6" x 2'-0"	(8) - #5 E.W. BOT.
F3	8'-0" x 8'-0" x 2'-0"	(9) - #5 E.W. BOT.
F4	8'-0" x 8'-0" x 1'-3"	(9) - #5 E.W. BOT.

PEDESTAL SCHEDULE		
MARK	SIZE	REINFORCEMENT
P1	2'-0" x 2'-6"	(10) - #6 VERT. W/ #3 TIES @ 8" O.C.

WALL FOOTING SCHEDULE		
MARK	SIZE (W x D)	REINFORCEMENT
WF1	2'-0" x 2'-0"	(2) - #5 CONT. T & B

COLUMN SCHEDULE		
MARK	SIZE	BASE PL
C1	HSS6x6x1/4	PL 1" x 12" x 1'-0"

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DRAWING
FOUNDATION PLAN

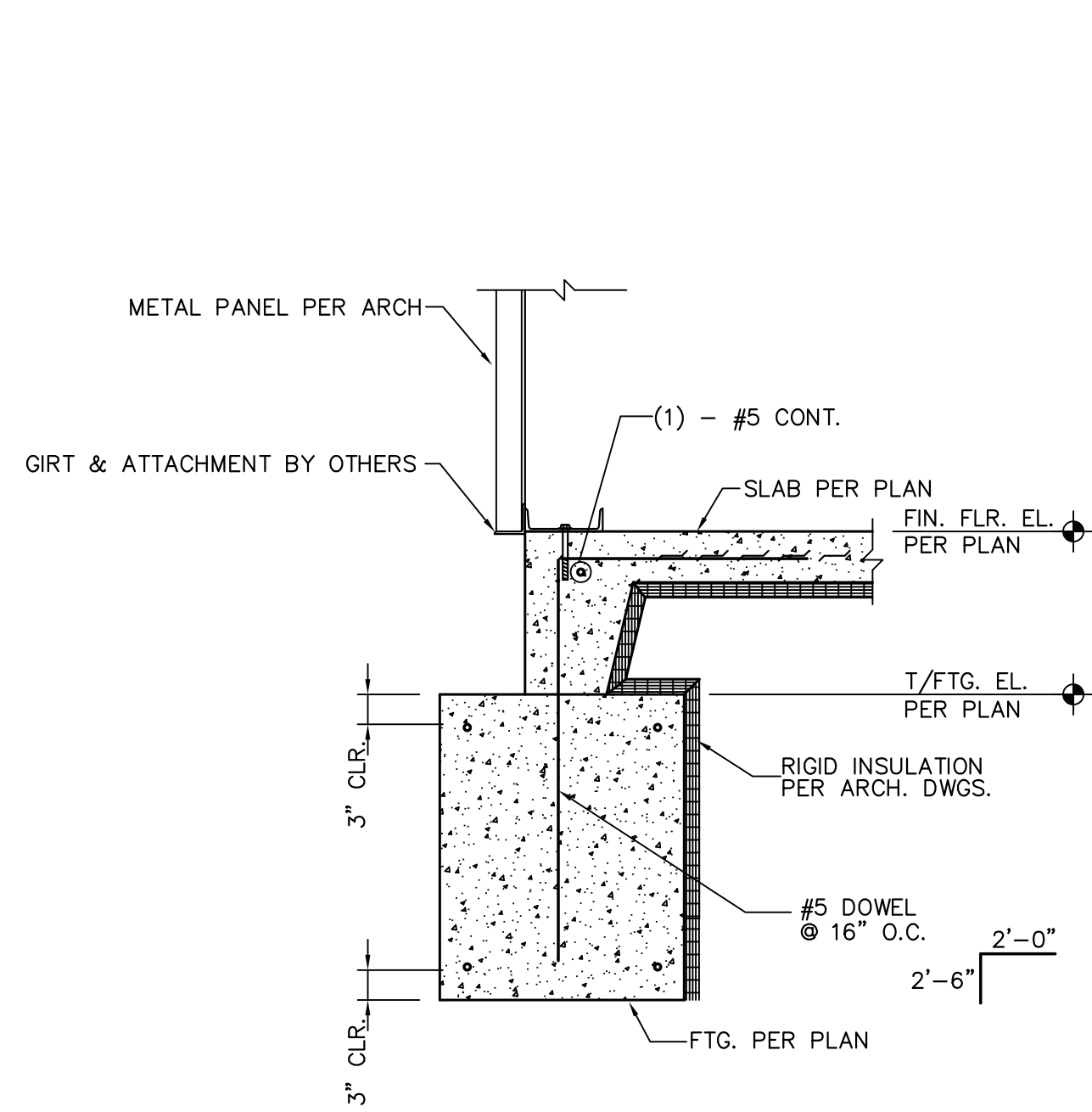
ISSUE
PRICING SET - 11/07/2025

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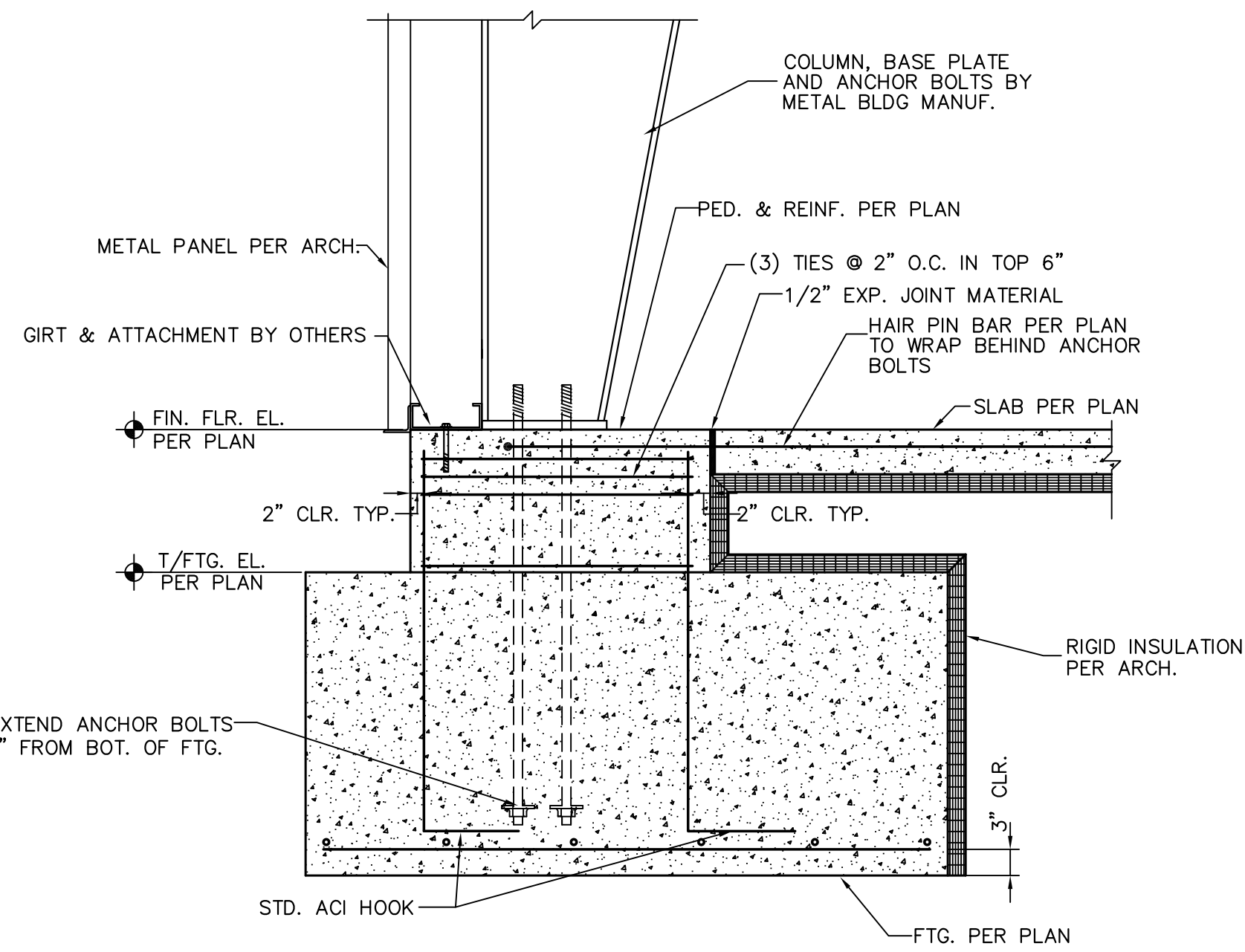
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DATE
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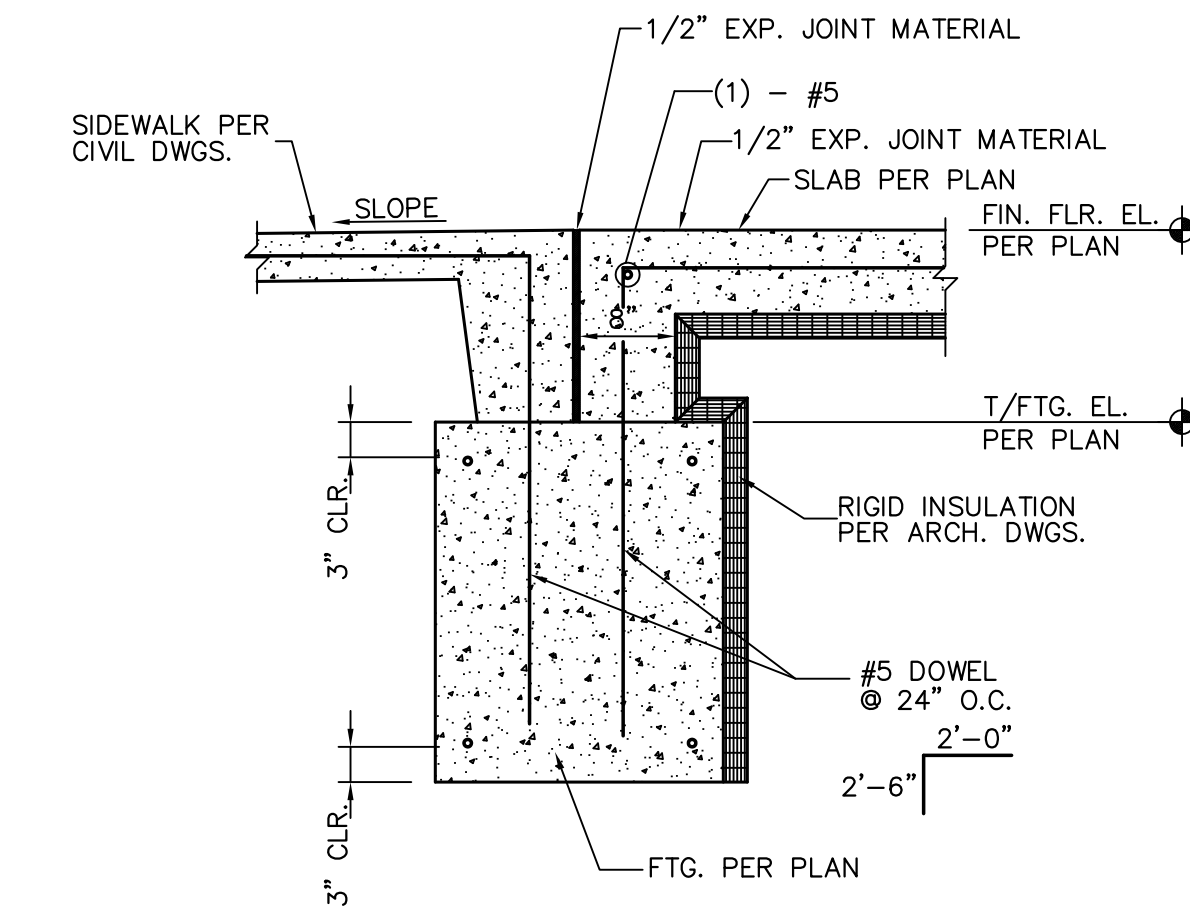
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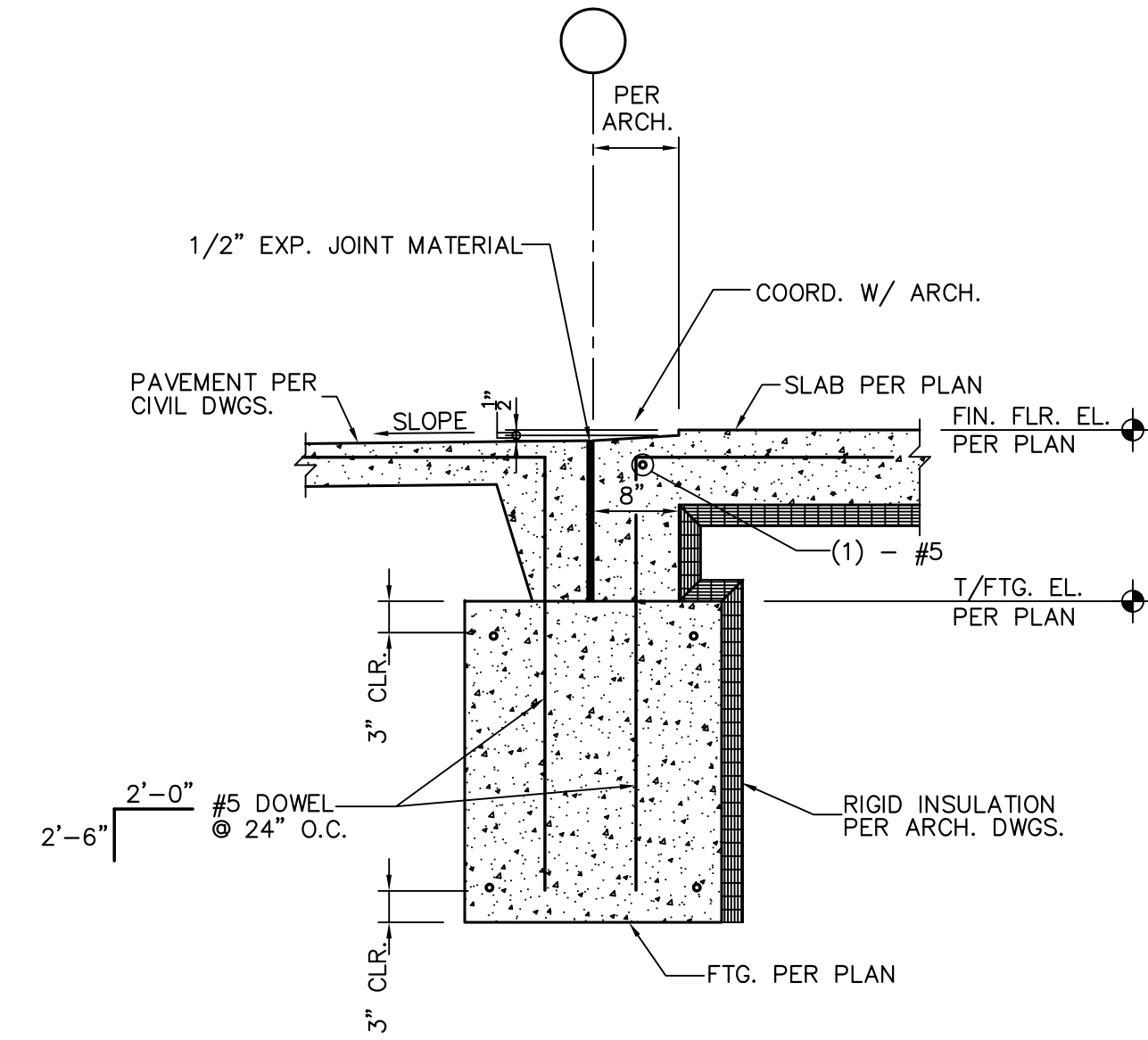
SECTION 1
F201



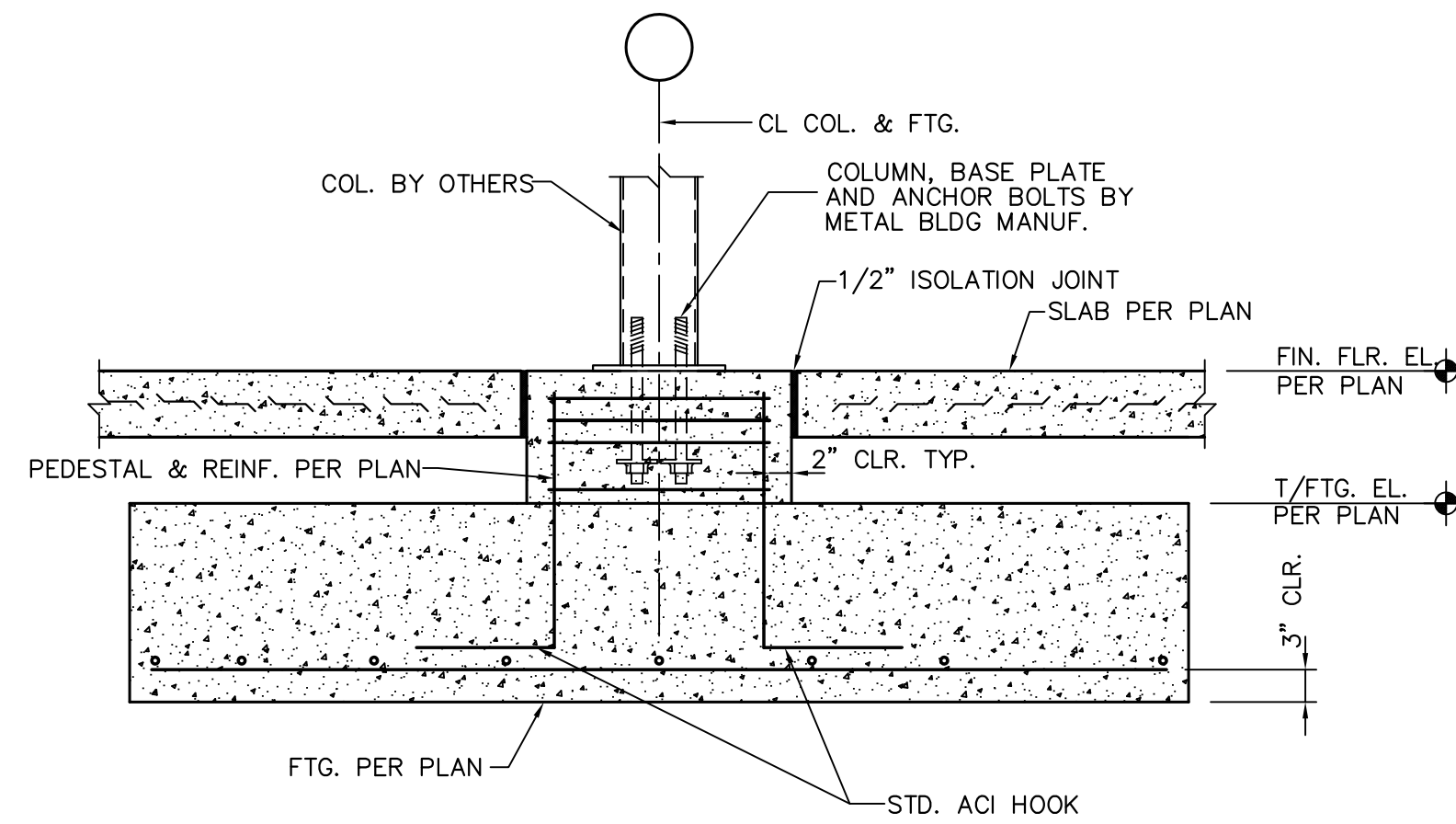
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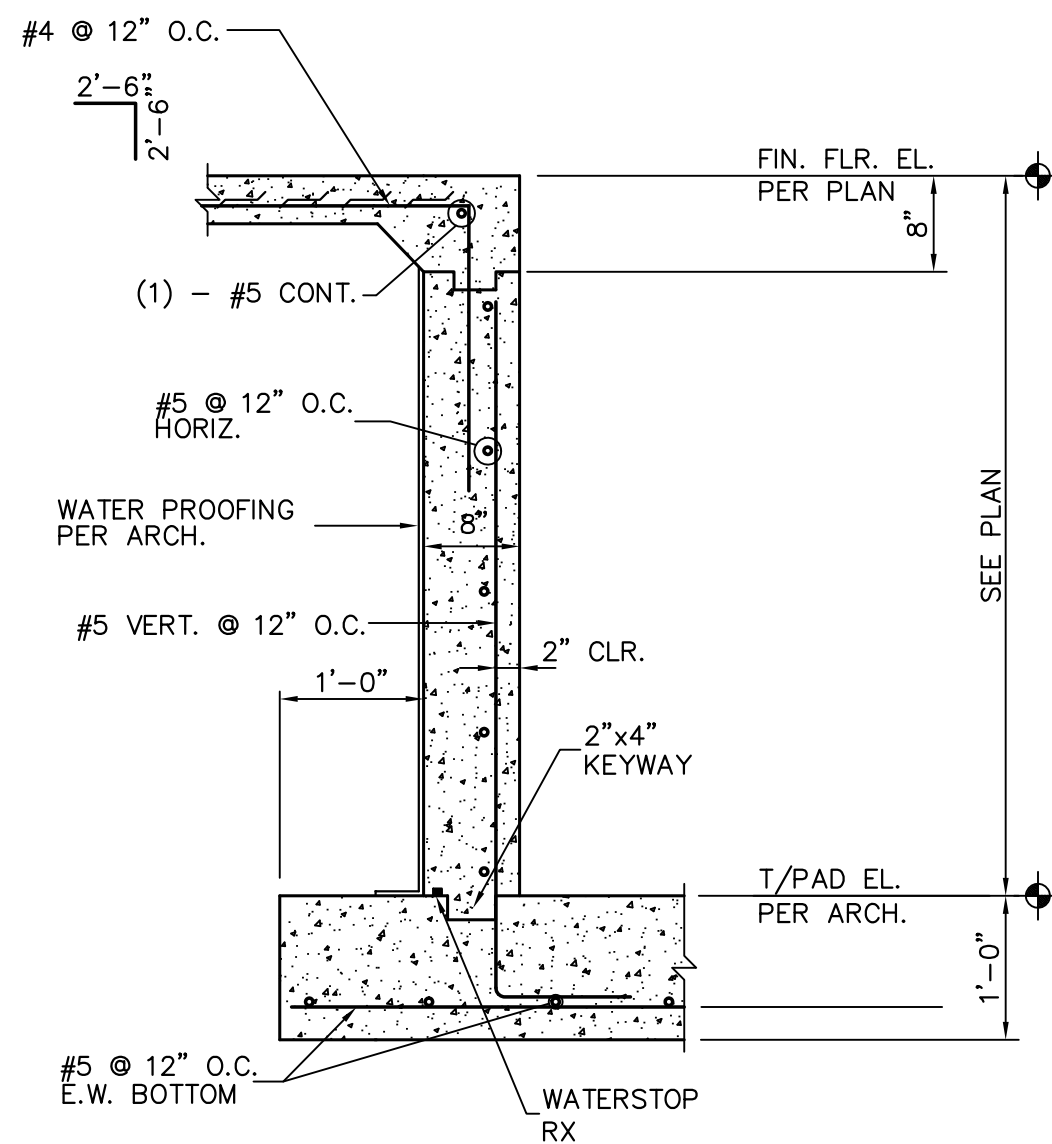
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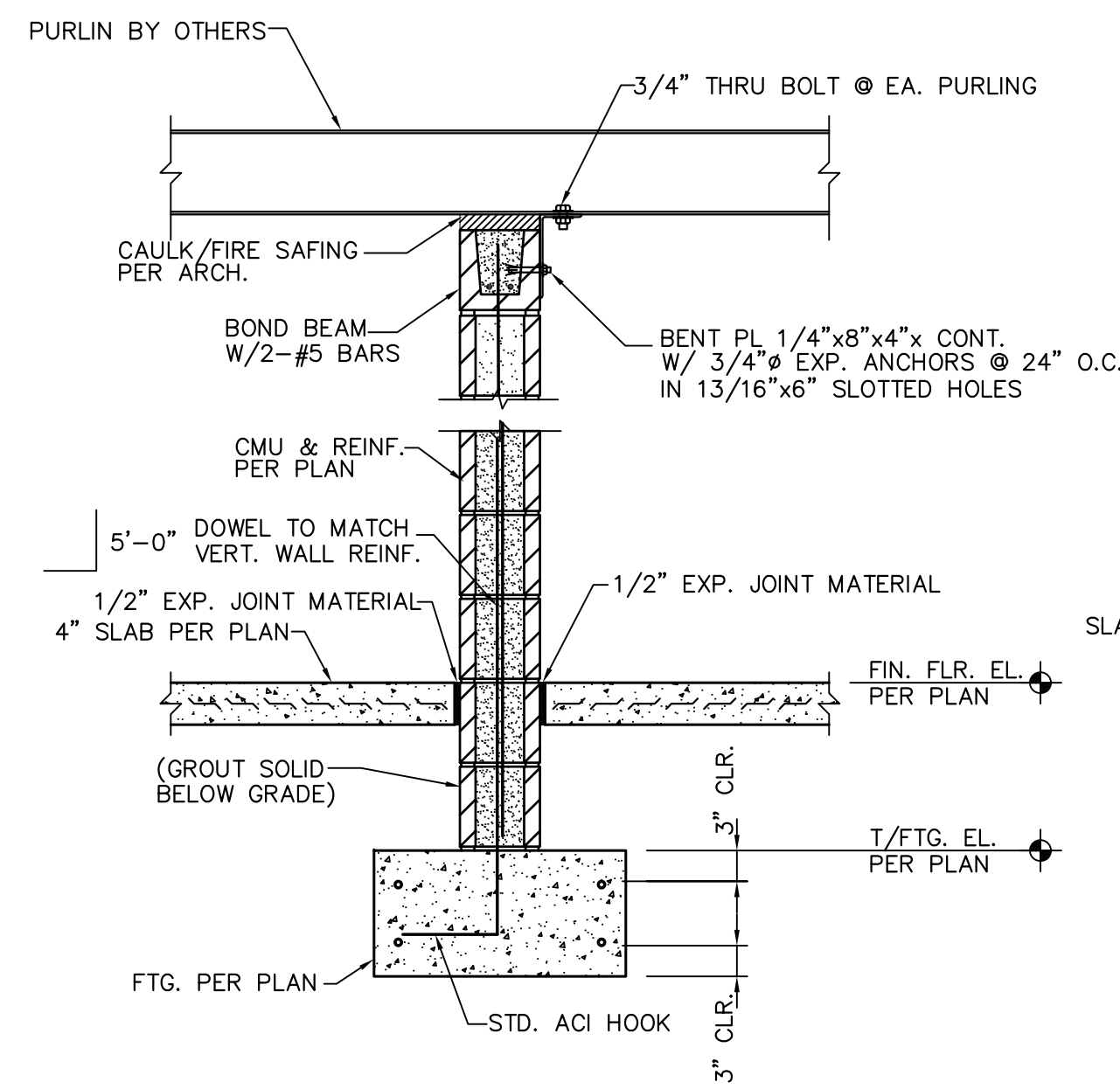
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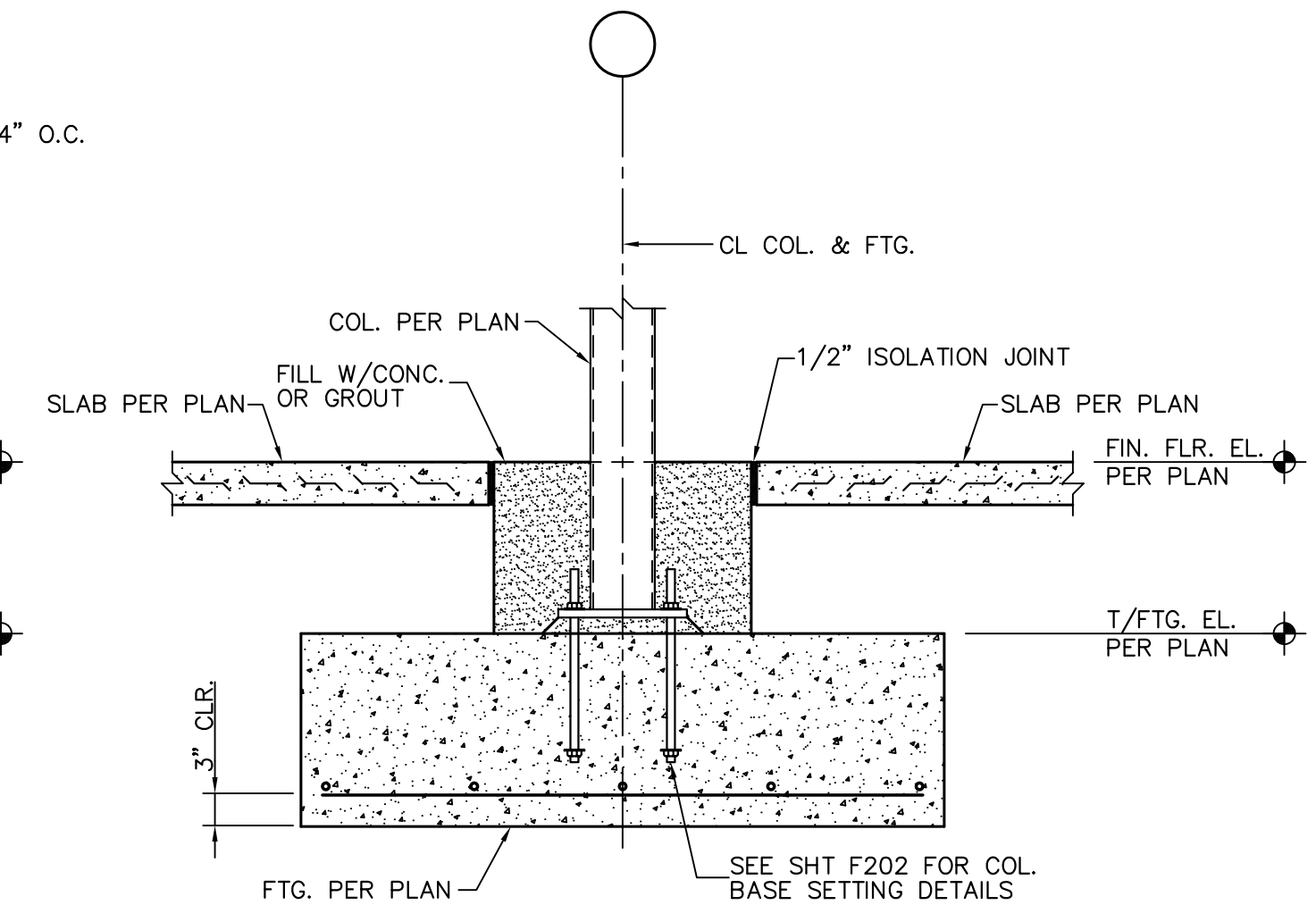
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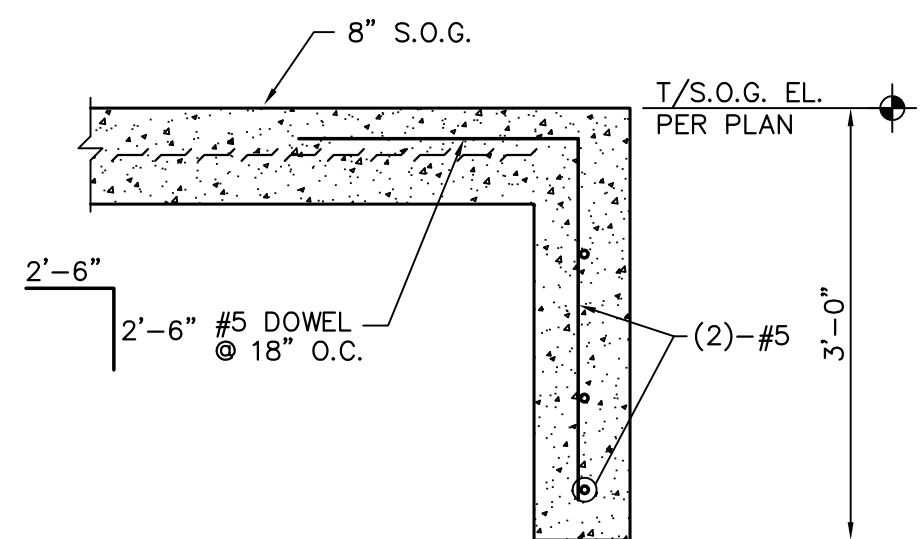
SECTION 6
F201



SECTION 7
F201



SECTION 8
F201



SECTION 9
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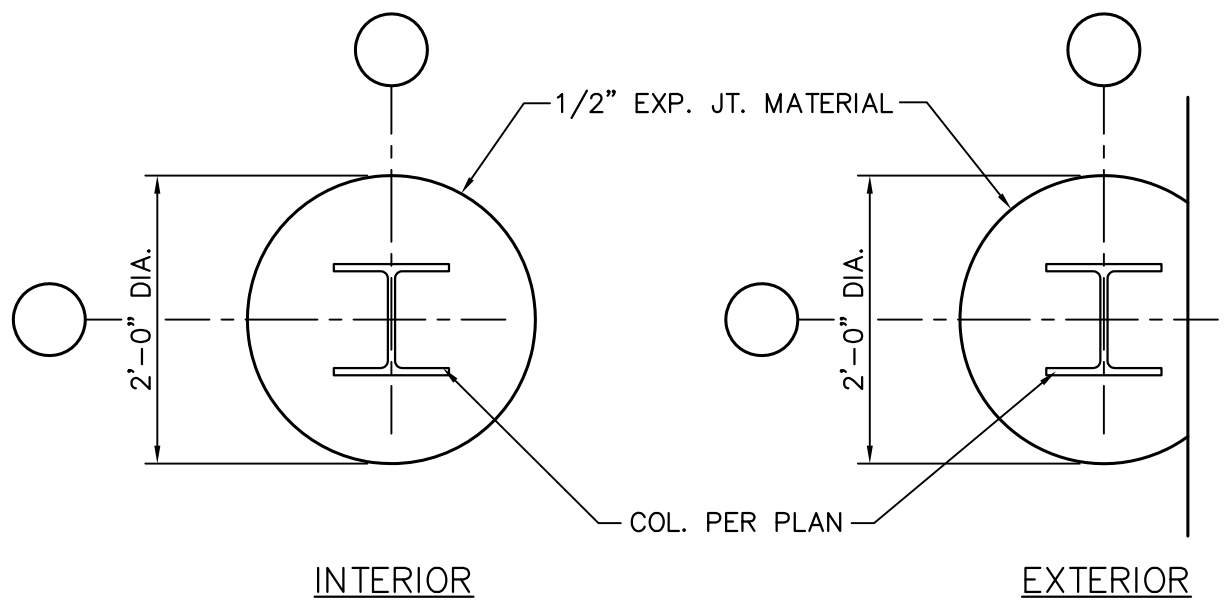
DRAWING
FOUNDATION DETAILS

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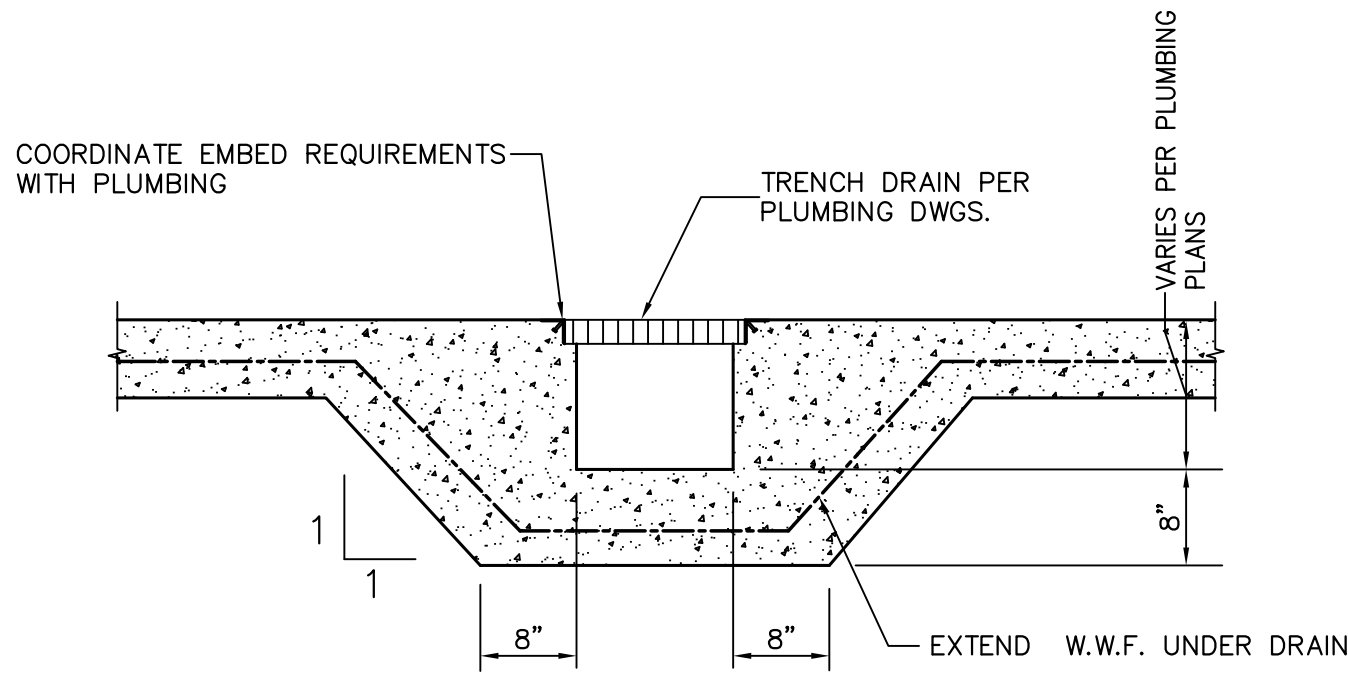
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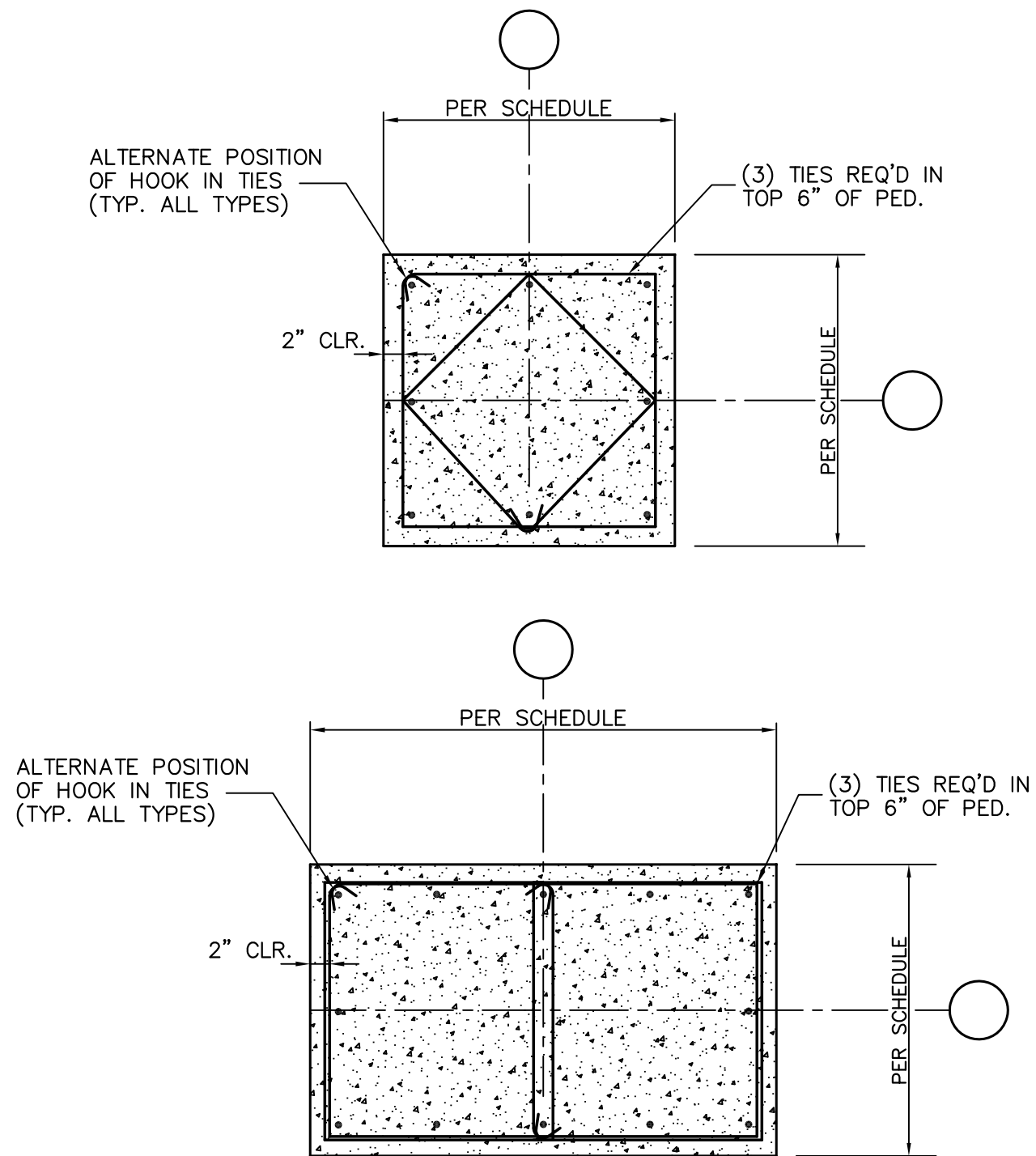
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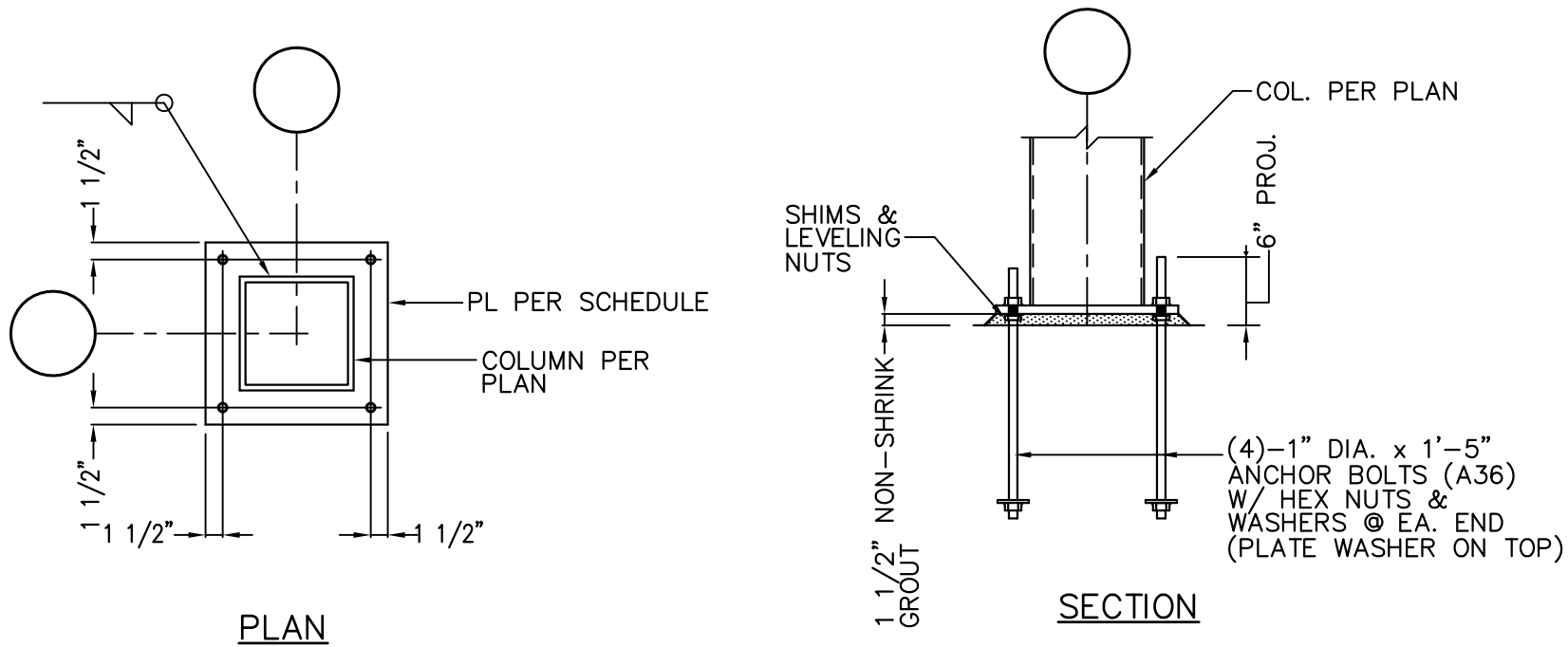
TYPICAL COLUMN ISOLATION JOINTS
NO SCALE



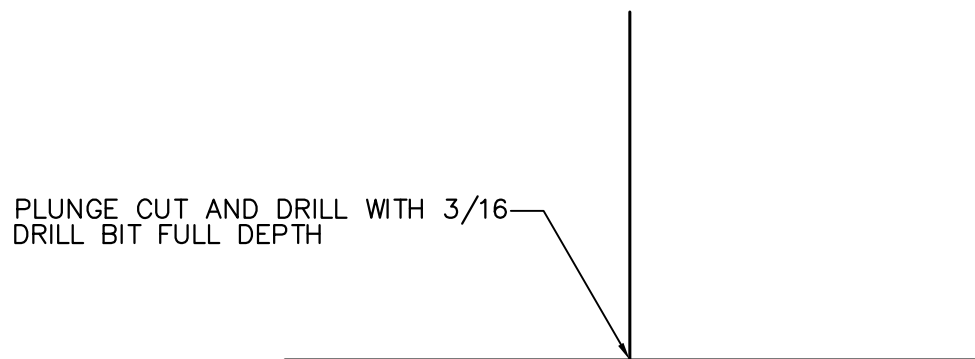
TYPICAL TRENCH DRAIN DETAIL



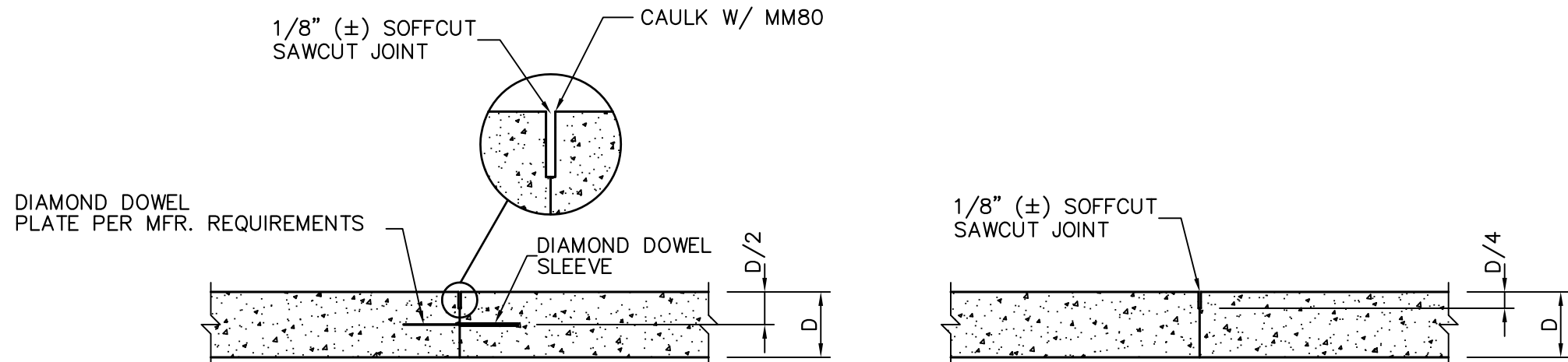
TYPICAL PEDESTAL DETAIL



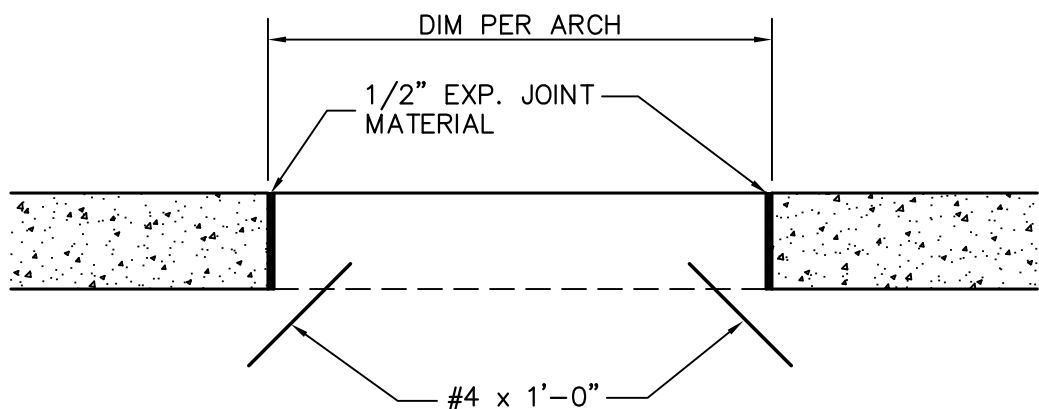
TYP. COLUMN BASE DETAIL
NO SCALE



TERMINAL JOINT DETAIL



TYPICAL SLAB ON GRADE JOINTS
NO SCALE



DETAIL AT DOOR JAMBS
SCALE: 3/4" = 1'-0"

GENERAL

Reference Architectural, Mechanical, Electrical, and Plumbing drawings for openings, sleeves, curbs, inserts, etc. not herein indicated. All sleeves or openings required in structural members shall be approved by the Structural Engineer.

The structure has been designed for the final, in-service condition. It shall be the contractor's responsibility through methods and means of construction to maintain the integrity of the structure during all stages of construction and ensure safe working conditions.

The Contractor shall be responsible for preparation of shop drawings to enable fabrication, erection and construction all parts of the work in accordance with the drawings and specifications. Shop drawing review by the Structural Engineer shall be for design conformance only. The Contractor shall be responsible for all dimensions, accuracy, and fit of work.

FOUNDATIONS

Exterior foundations shall bear a minimum of 3'-0" below finish grade and shall bear on undisturbed soil.

Allowable Soil bearing pressures: 1,500 psf

Foundation excavations shall be made to plan elevations and inspected by a qualified Geotechnical Engineer. Soil conditions found to be unacceptable shall be modified by one of the following procedures:

1. Remove all unacceptable soil and replace with compacted material as directed by the Geotechnical Engineer.
2. Lower foundations to an acceptable level and install concrete pedestals as directed by the Structural Engineer.

Foundation excavations shall be compacted to a level surface (or formed where soil conditions do not allow). The excavation shall be protected from weather if concrete placement does not occur within 24 hours of excavation of the footing.

CONCRETE

Reinforced concrete has been designed in conformance with the Building Code Requirements for Structural Concrete (ACI 318) and Commentary (ACI 318R).

Concrete mixing, transporting, and placing shall be in accordance with the latest edition of the Specifications for Structural Concrete for Buildings (ACI 301).

Concrete listed below shall be proportioned with sand fine aggregate and normal weight coarse aggregates (ACI 302.1R) and Type I Portland cement (ASTM C150). Compressive strength (f'c) at 28 days shall conform to the following:

Spread footings and wall footings 3,000 psi

Sidewalks and exterior slabs 4,000 psi (6%+/-1% air entrainment)

Slabs on grade 4,000 psi

Slab on grade construction shall be in conformance with the latest edition of the Guide for Concrete Floor and Slab Construction (ACI 302.1R) and the following minimum requirements:

Minimum cement content (interior uses) 5 sacks/cys
Minimum cement content (exterior uses) 6 sacks/cys
Maximum w/c ratio (interior uses) 0.48
Maximum w/c ratio (exterior uses) 0.45
Maximum slump prior to addition of plasticizers 3 inches
Maximum slump if plasticizers are not used 4 inches
(measured at point of discharge from pipe)

Do not finish surface until concrete has lost surface water sheen or all free water. Do not sprinkle cement on the slab surface.

Concrete slabs shall be cured immediately after finishing using a sprayed on liquid curing compound or other methods approved by the Structural Engineer.

Compressive strength tests shall conform to the requirements of ASTM C39. Specimens shall be taken in accordance with the latest edition of ASTM C31. Each set shall include one cylinder tested at 7 days, 2 cylinders tested at 28 days and one cylinder retained in reserve. Test results shall be forwarded to the Structural Engineer for review and comment. Protect the concrete surface from precipitation and on hot, dry, windy days use wet burlap, plastic membranes or fogging.

Flexural Strength Tests: ASTM C78, one set of 4 beams for each days pour, and for each 200 cu. yd., test two specimens at 14 days, one at 28 days and two at 56 days.

Construction joints between pours shall be prepared by roughening the contact surface to an amplitude of approximately 1/4 inch, removing all surface laitance.

The contractor shall submit mix designs for each proposed class of concrete for the Structural Engineer to review.

REINFORCING STEEL

Reinforcing steel shall be deformed bars of new billet steel with a minimum yield strength of 60,000 psi conforming to ASTM A615.

Detailing, fabricating, and placing of reinforcement shall be in accordance with the latest edition of the following standards: Specifications for Structural Concrete for Buildings (ACI 301), ACI Detailing Manual(SP66).

Reinforcing bars shall not be welded, heated or cut unless shown on the contract documents.

Welded wire fabric shall be smooth wire fabric (ASTM A185), supplied in flat sheets.

Welded wire fabric shall be placed 2" down from the top of the slab for slabs on grade and lapped a minimum of 14 inches on all sides.

Concrete cover shall be as specified in the latest edition of ACI 318, unless notes otherwise on plan. Use standard bar chairs and spacers to maintain concrete protection specified.

Splicing of reinforcing bars shall conform to latest edition of ACI 318. Use a Class "B" lap at splices unless indicated elsewhere.

MASONRY

Concrete masonry design conforms to the latest edition of the Building Code Requirements for Concrete Masonry Structures (ACI 530).

Concrete masonry construction shall be in accordance with the latest edition of the Specification for Concrete Masonry Construction (ACI 530.1).

Minimum compressive strength (f'm) for concrete masonry units shall be 1,500 psi at 28 days.

Mortar mixes (type M below grade, type M or S above grade) shall be proportioned to meet the requirements of ASTM C270 or C476.

Masonry walls shall be laterally braced in accordance with all applicable codes and design requirements until roof construction is permanently installed.

MECHANICAL ANCHORS

Expansion and sleeve anchors shall be carbon steel anchors as manufactured by Hilti Fastening Systems or approved equal.

Anchors shall be installed per manufacturer's recommendations.

Anchors shall not be installed in concrete until it has attained its specified minimum 28 day compressive strength (f'c).

GROUT FOR COLUMN BASEPLATES

Grout for column base plates shall be a nonmetallic, non-shrink grout (complying with the requirements of ASTM C827 or CRD-C621), premixed, non-corrosive, non-staining product containing Portland Cement, silica sands, and fluidity improving compounds. Minimum compressive strength (f'c) at 28 days shall be 4,000 psi.

Compressive strength tests shall conform to the requirements of ASTM C109.

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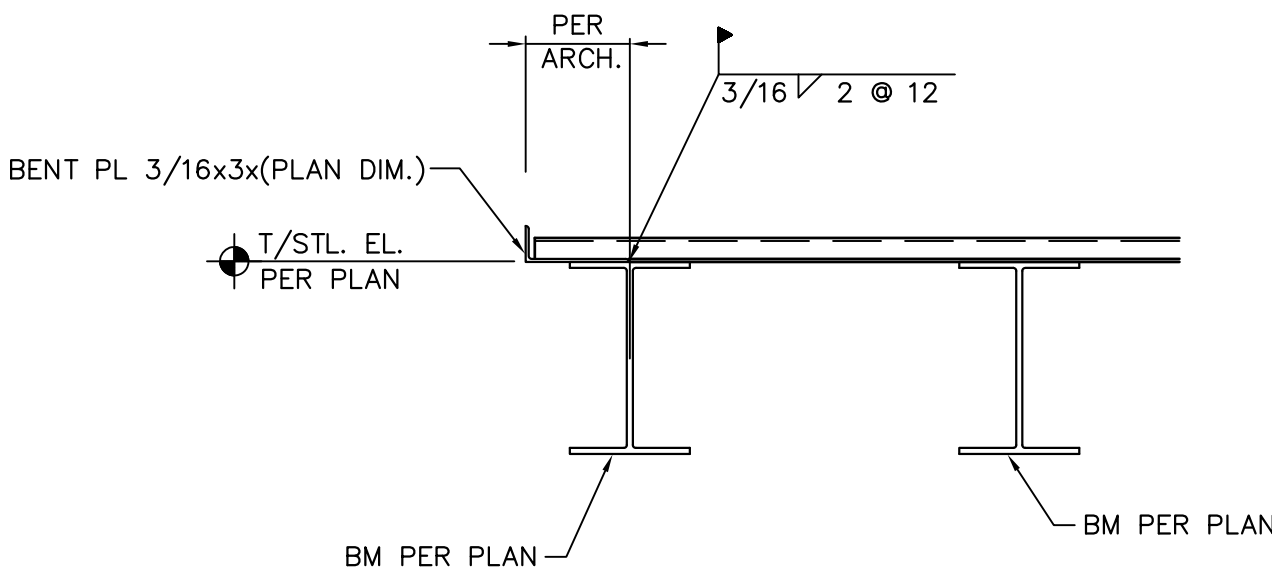
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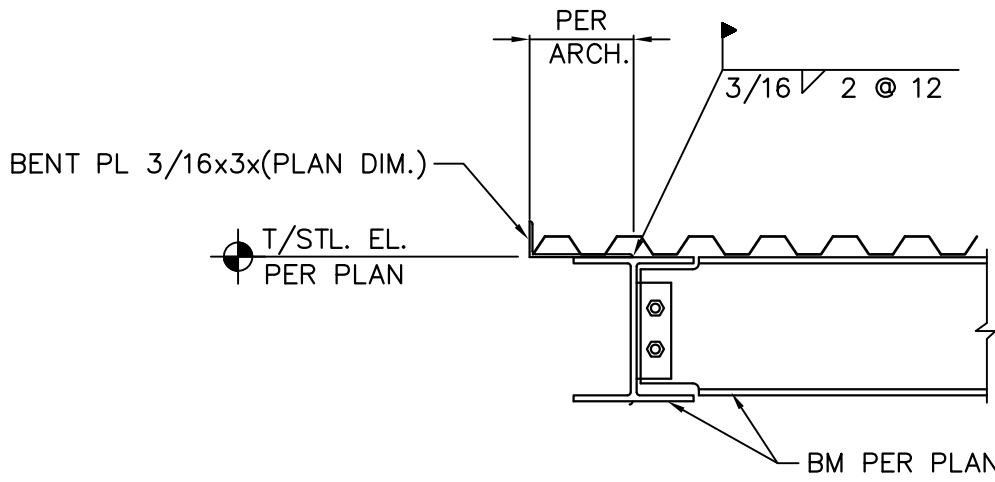
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DRAWING	FOUNDATION DETAILS & NOTES
ISSUE	
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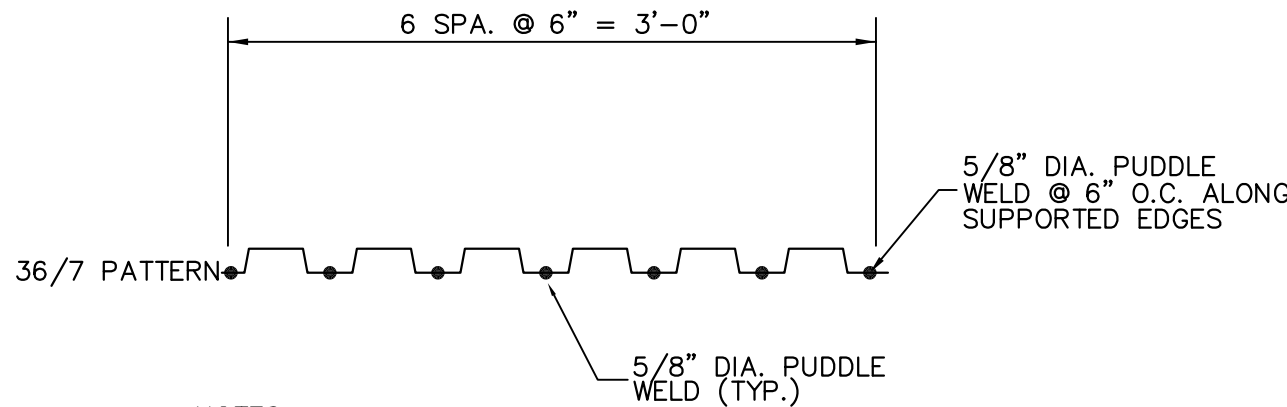
F202



SECTION 1
S201



SECTION 2
S201



- NOTES:
- 1) SIDELAP FASTENERS TO CONSIST OF 2-#10 TEK SCREWS PER SPAN
 - 2) ALL ROOF DECK TO BE 22 GAGE TYPE B (WIDE RIB) METAL DECK
 - 3) SUPPLY IN SHEETS LENGTHS TO PROVIDE A MINIMUM 3-SPAN CONDITION

TYPICAL ROOF DECK
ATTACHMENT PATTERN

STRUCTURAL STEEL

Detailing, fabrication and erection of structural steel shall be in conformance with the AISC Specification for Structural Steel Buildings, latest edition with amendments, and the AISC Code of Standard Practice for Steel Buildings and Bridges, latest edition with amendments.

Rolled shapes shall conform to ASTM A992, Grade 50.

Plates shall conform to ASTM A36.

Tubing shall conform to ASTM A500, Grade B.

Column anchor bolts shall conform to ASTM A36, unless noted otherwise.

Rolled shapes and plates shall be shop-painted gray with a rust inhibiting primer, unless directed otherwise by the specification. Touch up all nicks and abrasions after erection is complete.

Design connections not shown per the latest edition of the AISC Specification and Manual of Steel Construction.

Bolted connections shall be made with 3/4" diameter bolts, conforming to ASTM A325, tightened to the snug tight condition, unless noted otherwise. Bolted connections in wind brace elements shall be tightened using the turn-of-nut method. Connections shall conform to the Specification for Structural Joints Using ASTM A325 or A490 Bolts, approved by the Research Council on Structural Connections of the Engineering Foundation.

Welding of structural members shall be in conformance with the American Welding Society's (AWS) Structural Welding Codes for: Steel ANSI/AWS D1.1, Sheet Steel ANSI/AWS D1.3 and Reinforcing Steel ANSI/AWS D1.4.

Typical welded connections shall be made with E70XX electrodes. When ASTM A992 steel is used as a base metal, E70XX Low Hydrogen electrodes shall be used.

Base material within 2" of any field weld shall be free of materials that would prevent proper welding or produce toxic fumes during welding operations.

Splicing of structural steel members is not permitted without prior approval of the Structural Engineer.

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